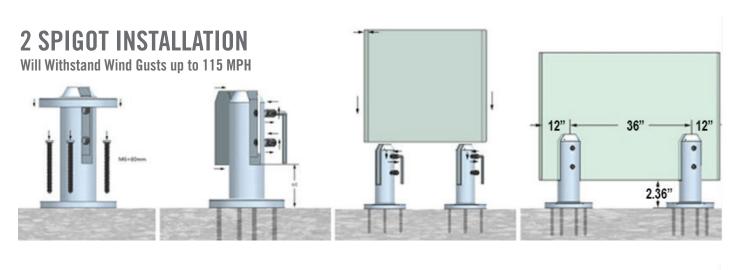
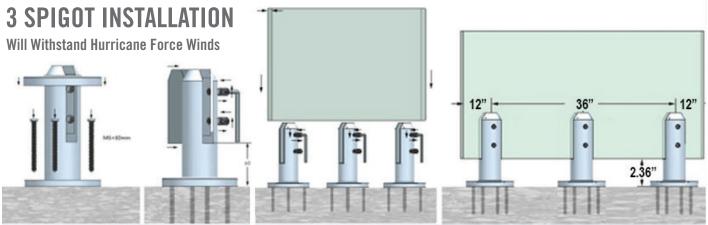


INSTALLATION WITH TWO OR THREE SPIGOTS





Wood Deck Installation

The hold down force for each spigot is 2,500 lbs. We suggest using 3/8" diameter x 3.5" ASTM A307 Grade A structural bolts as they have sufficient capacity to resist this force. Use with flat washer to fasten spigots to wood deck. ASTM A307 Grade A structural bolts and washer should be cadmium plated or stainless steel so they do not rust.

Lag bolts must be installed into rim joists or lam beam or properly blocked sub structure. If lag bolts are attached to deck planks only failure will occur as a result of improper installation. Improper installation and failure may result in injuries or death. Do it once and do it right!



Installation of Spigots Using Wood Planks

Installation of CVGR Spigots to wood planks are fine as long as you use three 2" x10"s. You must tie the three plies of wood together with (4) #8 4" wood screws*, located 3" from spigot screws.

More installation information including additional fastener product recommendation listed on next page. Product Specifications included as well.

*Or use four RSS Rugged Structural Screws (5/16x4) by GRK Fasteners shown at top of next page.



Fastener Recommendation

We recommend RSS Rugged Structural Screws by GRK Fasteners (product image shown). Based on their rated loads the screw has an allowable steel strength in tension of 1,415lb. Utilizing a 200lb point load at the top of a 42" glass panel with 2 spigots, the tensile load per fastener (1 per spigot) is 1,333lb. The GRK RSS 5/16x4 fasteners are strong enough to resist the applied load.

This product is available at Home Depot:

- Internet #203525067
- Model #112225
- Store SKU #518167

VERY IMPORTANT

- Apply a bit of lubricant (petrolium jelly) to the inside of the spigot's black plastic boot where the boot meets the
 glass. The application of lubrication prevents the spigot's black plastic boot from grabbing the glass panel
 should you slide/move the glass to adjust its position while in the spigots.
- Do not attempt to slide the glass panel while it is in the spigot if there is no lubricant on the black plastic spigot boot as the glass panel may stick and fail.
- If no lubrication is applied to the spigot boot, you must lift the glass panel out of the spigot, adjust its position as desired and then lower the glass panel back into the spigot.

Helpful Installation Tips

- Mark spigot location on glass panels with a crayon or wax marker. This allows for fast and easier installation of panel in proper location.
- Some clients have suggested screwing the rim joist to the joist as the screws will hold the rim joist tight to the joist where as nails may not.

Specifications

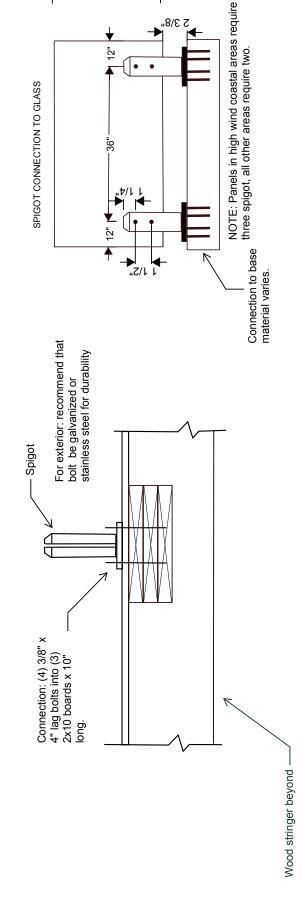




Part Number	CVGR 316 SSOD48-180 Satin Finish	
Product Name	Round Deck Mount Spigot	
Spigot Size	1.9" diameter x 7.1" tall	
Spigot Weight	5.5 lbs.	
Glass Thickness/ Dimensions/Weight (per panel)	1/2"/ 60" width x 39.37" height/105 lbs.	
Accessories Included	Base Cover, Rubber Gasket	

CVGR Spigot Attachment to Wood Deck

(additional instructions)



36" or 42".

12"

Part Number	CVGR 316 SSOD48-180 Satin Finish
Product Name	Round Deck Mount Spigot
Spigot Size	1.9" diameter x 7.1" tall
Spigot Weight	5.5 pounds
Glass Thickness/ Dimensions/Weight (per panel	13.5 mm/60" wide x 39.37" tall/ 98.5 pounds
Accessories Included	Base Cover, Rubber Gasket

CVGR 316 SSOD4	Part
Product Name Spigot	Prod
Spigot Size 1.9" diameter x 7.	Spigo
Spigot Weight 5.5 pounds	Spigo
Glass Thickness/ Dimensions/Weight 39.37" tall/ 98.5 pc (per panel	Glass Thic Dimensior (per panel
sories Base Cover, Rubt ed Gasket	Accessories Included

SIMULTANEOUS.

2. WHEN THREE SPIGOTS ARE USED, THIS PRODUCT COMPLIES WITH THE REQUIREMENTS FOR HIGH VELOCITY HURRICANE ZONES (HVHZ).

3. SPIGOTS TO BE 316 STAINLESS STEEL.

4. CONNECTION TO BUILDING VARY BY PROJECT, TO BE DESIGNED BY PROJECT ENGINEER. CONNECTION TO WOOD STAIR STRINGER IS

5. THE ABIILITY OF THE EXISTING HOST STRUCTURE TO SAFELY SUPPORT THE LODS SHALL BE DETERMINED BY THE PROJECT

DESIGNBED AND SHOWN HEREIN

ENGINEER. 6. MECHANICAL FASTENERS, AS SHOWN.

CONNECTION TO DECK SHOWN (4) 3/8" LAG BOLTS THROUGH DECK TO

FULLY TEMPERED OR HEAT-TREATED GLASS"

(3) 2X10 BOARDS, MEETS IBC DECK REQUIREMENTS FOR A 200 POUND POINT LOAD OR 500 POUND LINE LOAD AT ANY PLACE ON PANEL.

LOADING TO BE EITHER VERTICAL OR HORIZONTAL BUT NOT

INTERNATIONAL BUILDING CODE (IBC) SECTION 1607.8. IT IS ALSO IN ACCORDANCE WITH 2015 IBC SECTION 2407 THAT REQUIRES ALL-GLASS HANDRAILS AND GUARDS BE "LAMINATED GLASS CONSTRUCTED OF THIS GUARDRAIL SYSTEM IS IN COMPLIANCE WITH THE 2014 AND 2017

GENERAL NOTES:

ClearView Glass Railings Attachment to Concrete

CVGRailings spigot baseplate is about 4" diameter. With this, we have come up with the following:

We are specifying an adhesive anchor system by HILTI: 3/8" diameter HIT-Z anchor, with their HY200-R adhesive. Effective embedment = 2 3/8". HILTI has many anchors and it is important that they use this exact anchor. We have attached the HILTI report that describes this design. It is important that they closely follow the installation steps, especially the hole preparation: the most common failure mechanism is a lack of bond between the adhesive and the concrete because the installer did not remove all dust within the hole before injecting the adhesive. A lack of correct preparation will void these calculations and HILTI's support of their anchor. This is important.

This design assumes a 3.15" spacing between anchors, into a concrete slab. Anchors to be at least 6" from all embedded PT cables and from the edge of the concrete slab. The location of the cables to be determined by others.

Note that the loads shown in the report come from my computer modeling of a 200 pound/ft (plf) horizontal line load applied to the top of the 42" tall panel. As a reminder, the IBC requires guardrails be designed to resist a 50 plf horizontal line load @ the top of the panel; the code has increased this by a factor of 4 for all-glass panels such as yours, hence, the 200 plf.

The installer of a PT deck can order the anchor and adhesive directly from HILTI through their website if they don't have a local rep (Home Depot carries HILTI products).



Chris Hartnett, PE*, LEED AP
Principal Engineer
*MN, WI

<u>chartnett@amengtest.com</u> **D:** 651.647-2750 | **C:** 612.503-0048 550 Cleveland Avenue North St. Paul, MN 55114



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Design: Concrete - Apr 5, 2021 Date: 4/5/2021
Fastening point:

Specifier's comments:

1 Input data

Anchor type and diameter: HIT-HY 200 + HIT-Z 3/8

Item number: 2018440 HIT-Z 3/8" x 4 3/8" (element) / 2022793 HIT-HY

200-R (adhesive)

Effective embedment depth: $h_{ef,opti} = 2.375 \text{ in. } (h_{ef,limit} = 4.500 \text{ in.})$

Material: DIN EN ISO 4042

Evaluation Service Report: ESR-3187

Issued I Valid: 4/1/2020 | 3/1/2022

Proof: Design Method ACI 318-08 / Chem Stand-off installation: $e_h = 0.000$ in. (no stand-off); t = 0.500 in.

Anchor plate^R: $I_x \times I_y \times t = 6.000$ in. $\times 6.000$ in. $\times 0.500$ in.; (Recommended plate thickness: not calculated)

Profile: Round bars (AISC), 2 1/2; (L x W x T) = 2.500 in. x 2.500 in.

Base material: cracked concrete, 4000, f_c ' = 4,000 psi; h = 8.000 in., Temp. short/long: 32/32 °F

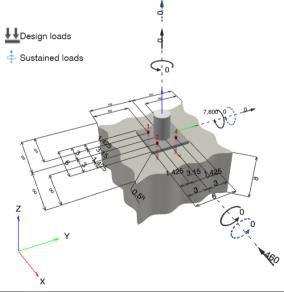
Installation: hammer drilled hole, Installation condition: Dry

Reinforcement: tension: condition B, shear: condition B; no supplemental splitting reinforcement present

edge reinforcement: none or < No. 4 bar

Seismic loads (cat. C, D, E, or F) no

Geometry [in.] & Loading [lb, in.lb]





Input data and results must be checked for conformity with the existing conditions and for plausibility! PROFIS Engineering (c) 2003-2021 Hilti AG, FL-9494 Schaan Hilti is a registered Trademark of Hilti AG, Schaan

^R - The anchor calculation is based on a rigid anchor plate assumption.



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Fastening point:

1.1 Design results

Case	Description	Forces [lb] / Moments [in.lb]	Seismic	Max. Util. Anchor [%]
1	Combination 1	$N = 0$; $V_x = -460$; $V_y = 0$;	no	51
		$M_x = 0$; $M_y = 7,800$; $M_z = 0$;		
		$N_{sis} = 0$; $M_{x sis} = 0$; $M_{y sis} = 0$;		

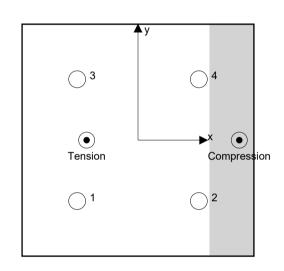
2 Load case/Resulting anchor forces

Anchor reactions [lb]

Tension force: (+Tension, -Compression)

Anchor	Tension force	Shear force	Shear force x	Shear force y
1	910	115	-115	0
2	77	115	-115	0
3	910	115	-115	0
4	77	115	-115	0

max. concrete compressive strain: 0.13 [%] max. concrete compressive stress: 580 [psi] resulting tension force in (x/y)=(-1.329/0.000): 1,974 [lb] resulting compression force in (x/y)=(2.622/0.000): 1,974 [lb]



2

Anchor forces are calculated based on the assumption of a rigid anchor plate.

3 Tension load

	Load N _{ua} [lb]	Capacity • N _n [lb]	Utilization $\beta_N = N_{ua}/\Phi N_n$	Status
Steel Strength*	910	4,749	20	OK
Pullout Strength*	910	5,169	18	OK
Sustained Tension Load Bond Strength*	N/A	N/A	N/A	N/A
Concrete Breakout Failure**	1,974	3,874	51	OK

^{*} highest loaded anchor **anchor group (anchors in tension)



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Fastening point:

3.1 Steel Strength

 $\begin{aligned} & \text{N}_{\text{sa}} &= \text{ESR value} & \text{refer to ICC-ES ESR-3187} \\ & \phi & \text{N}_{\text{sa}} \geq \text{N}_{\text{ua}} & \text{ACI 318-08 Eq. (D-1)} \end{aligned}$

Variables

A_{se,N} [in.²] f_{uta} [psi] 0.08 94,200

Calculations

N_{sa} [lb] 7,306

Results

 $\frac{N_{sa}[lb]}{7,306}$ $\frac{\phi}{steel}$ $\frac{\phi}{N_{sa}[lb]}$ $\frac{N_{ua}[lb]}{910}$

3.2 Pullout Strength

 $\begin{array}{ll} N_{pn} &= N_p & \text{refer to ICC-ES ESR-3187} \\ \varphi & N_{pn} \geq N_{ua} & \text{ACI 318-08 Eq. (D-1)} \end{array}$

Variables

N_p [lb] 7,952

Calculations

-

Results

 N_{pn} [lb] $\Phi_{concrete}$ Φ_{pn} [lb] N_{ua} [lb] 0.650 0.650 0.650 0.650 0.650

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Fastening point:

3.3 Concrete Breakout Failure

N _{cbg} =	$\left(\frac{A_{Nc}}{A_{Nc0}}\right)$	$\psi_{\text{ ec},N} \ \psi_{\text{ed},N} \ \psi_{\text{c},N} \ \psi_{\text{cp},N} \ N_{\text{b}}$	ACI 318-08 Eq. (D-5)
--------------------	---------------------------------------	--	----------------------

$$\phi N_{cbg} \ge N_{ua}$$
 ACI 318-08 Eq. (D-1)

see ACI 318-08, Part D.5.2.1, Fig. RD.5.2.1(b)

$$A_{Nc0} = 9 h_{ef}^2$$
 ACI 318-08 Eq. (D-6)

$$\psi_{\text{ec,N}} = \left(\frac{1}{1 + \frac{2 e_{\text{N}}}{3 h_{\text{ef}}}}\right) \le 1.0$$
 ACI 318-08 Eq. (D-9)

$$\psi_{\text{ed,N}} = 0.7 + 0.3 \left(\frac{c_{\text{a,min}}}{1.5\text{h}} \right) \le 1.0$$
 ACI 318-08 Eq. (D-11)

$$\begin{array}{ll} \psi_{\,ed,N} &= 0.7 + 0.3 \left(\frac{c_{a,min}}{1.5h_{ef}}\right) \leq 1.0 & \text{ACI 318-08 Eq. (D-11)} \\ \psi_{\,cp,N} &= \text{MAX} \left(\frac{c_{a,min}}{c_{ac}}, \frac{1.5h_{ef}}{c_{ac}}\right) \leq 1.0 & \text{ACI 318-08 Eq. (D-13)} \\ N_b &= k_c \ \lambda \ \sqrt{f_c} \ h_{ef}^{1.5} & \text{ACI 318-08 Eq. (D-7)} \end{array}$$

$$N_b = k_c \lambda \sqrt{f_c h_{ef}^{1.5}}$$
 ACI 318-08 Eq. (D-7)

Variables

 h _{ef} [in.]	e _{c1,N} [in.]	e _{c2,N} [in.]	c _{a,min} [in.]	$\psi_{c,N}$
2.375	1.329	0.000	∞	1.000
c _{ac} [in.]	k _c	λ	f _c [psi]	
3.563	17	1	4,000	

Calculations

A _{Nc} [in. ²]	A _{Nc0} [in. ²]	$\Psi_{\text{ec1,N}}$	$\psi_{\text{ec2},\text{N}}$	$\psi_{\text{ed},N}$	$\psi_{\text{cp},\text{N}}$	N _b [lb]
105.58	50.77	0.728	1.000	1.000	1.000	3,935

Results

N _{cbg} [lb]	$\phi_{ m concrete}$	φ N _{cbg} [lb]	N _{ua} [lb]
5,960	0.650	3,874	1,974

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4 Shear load

	Load V _{ua} [lb]	Capacity V _n [lb]	Utilization $\beta_V = V_{ua}/\Phi V_n$	Status
Steel Strength*	115	1,929	6	OK
Steel failure (with lever arm)*	N/A	N/A	N/A	N/A
Pryout Strength (Concrete Breakout Strength controls)**	460	5,729	9	OK
Concrete edge failure in direction **	N/A	N/A	N/A	N/A

4.1 Steel Strength

 $\begin{array}{ll} {\rm V_{sa}} & = {\rm ESR} \ {\rm value} & {\rm refer} \ {\rm to} \ {\rm ICC\text{-}ES} \ {\rm ESR\text{-}}3187 \\ \varphi \ {\rm V_{steel}} \ge {\rm V_{ua}} & {\rm ACI} \ 318\text{-}08 \ {\rm Eq.} \ ({\rm D\text{-}}2) \end{array}$

Variables

A _{se,V} [in. ²]	f _{uta} [psi]	$lpha_{ m V,seis}$
0.08	94.200	1.000

Calculations

V_{sa} [lb] 3,215

Results

V _{sa} [lb]	ϕ_{steel}	φ V _{sa} [lb]	V _{ua} [lb]
3,215	0.600	1,929	115

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4.2 Pryout Strength (Concrete Breakout Strength controls)

$V_{cpg} = K_{cp} \left[\left(\frac{A_{Nc}}{A_{Nc0}} \right) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \right]$	ACI 318-08 Eq. (D-31)
$\phi V_{cpg} \ge V_{ua}$	ACI 318-08 Eq. (D-2)
A _{Nc} see ACI 318-08, Part D.5.2.1, Fig. RD.5.2.1(b)	
$A_{Nc0} = 9 h_{ef}^2$	ACI 318-08 Eq. (D-6)
$ \psi_{\text{ec,N}} = \left(\frac{1}{1 + \frac{2 e_{\text{N}}}{3 h_{\text{ef}}}} \right) \le 1.0 $	ACI 318-08 Eq. (D-9)
$\psi_{\text{ed,N}} = 0.7 + 0.3 \left(\frac{c_{a,\text{min}}}{1.5h_{\text{ef}}} \right) \le 1.0$	ACI 318-08 Eq. (D-11)
$\psi_{cp,N} = MAX\left(\frac{c_{a,min}}{c_{ac}}, \frac{1.5h_{ef}}{c_{ac}}\right) \le 1.0$	ACI 318-08 Eq. (D-13)

Variables

k_cp	h _{ef} [in.]	e _{c1,N} [in.]	e _{c2,N} [in.]	c _{a,min} [in.]
1	2.375	0.000	0.000	∞
$\psi_{c,N}$	c _{ac} [in.]	k _c	λ	f _c [psi]
1.000	3.563	17	1	4,000

Calculations

A _{Nc} [in. ²]	A _{Nc0} [in. ²]	$\psi_{\text{ ec1,N}}$	$\psi_{\text{ec2},N}$	$\psi_{\text{ed},N}$	$\psi_{\text{cp},N}$	N _b [lb]
105.58	50.77	1.000	1.000	1.000	1.000	3,935

ACI 318-08 Eq. (D-7)

Results

V _{cpg} [lb]	φ _{concrete}	φ V _{cpg} [lb]	V _{ua} [lb]	
8.184	0.700	5.729	460	

5 Combined tension and shear loads

β_{N}	$\beta_{\sf V}$	ζ	Utilization $\beta_{N,V}$ [%]	Status	
0.510	0.080	5/3	35	OK	

$$\beta_{NV} = \beta_N^{\zeta} + \beta_V^{\zeta} \le 1$$



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6 Warnings

- The anchor design methods in PROFIS Engineering require rigid anchor plates per current regulations (AS 5216:2018, ETAG 001/Annex C, EOTA TR029 etc.). This means load re-distribution on the anchors due to elastic deformations of the anchor plate are not considered the anchor plate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the design loading. PROFIS Engineering calculates the minimum required anchor plate thickness with CBFEM to limit the stress of the anchor plate based on the assumptions explained above. The proof if the rigid anchor plate assumption is valid is not carried out by PROFIS Engineering. Input data and results must be checked for agreement with the existing conditions and for plausibility!
- Condition A applies where the potential concrete failure surfaces are crossed by supplementary reinforcement proportioned to tie the potential concrete failure prism into the structural member. Condition B applies where such supplementary reinforcement is not provided, or where pullout or pryout strength governs.
- Design Strengths of adhesive anchor systems are influenced by the cleaning method. Refer to the INSTRUCTIONS FOR USE given in the Evaluation Service Report for cleaning and installation instructions.
- The present version of the software does not account for special design provisions for overhead applications. Refer to related approval (e.g. section 4.1.1 of the ICC-ESR 2322) for details.
- For additional information about ACI 318 strength design provisions, please go to https://submittals.us.hilti.com/PROFISAnchorDesignGuide/

Fastening meets the design criteria!



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7 Installation data

Profile: Round bars (AISC), 2 1/2; (L x W x T) = 2.500 in. x 2.500 in.

Hole diameter in the fixture (pre-setting) : $d_f = 0.438$ in.

Hole diameter in the fixture (through fastening): $d_f = 0.500$ in.

Plate thickness (input): 0.500 in.

Recommended plate thickness: not calculated

Drilling method: Hammer drilled

Cleaning: Compressed air cleaning of the drilled hole according to instructions

for use is required

Anchor type and diameter: HIT-HY 200 + HIT-Z 3/8 Item number: 2018440 HIT-Z 3/8" x 4 3/8" (element) /

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4/5/2021

2022793 HIT-HY 200-R (adhesive)
Maximum installation torque: 177 in.lb
Hole diameter in the base material: 0.438 in.
Hole depth in the base material: 2.375 in.

Minimum thickness of the base material: 4.625 in.

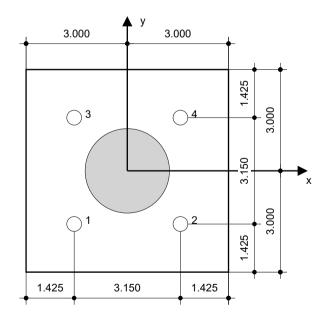
3/8 Hilti HIT-Z Carbon steel non-cleaning bonded expansion anchor with Hilti HIT-HY 200 Safe Set System

7.1 Recommended accessories

Drilling Cleaning Setting

- · Suitable Rotary Hammer
- · Properly sized drill bit

- · Dispenser including cassette and mixer
- · Torque wrench



Coordinates Anchor [in.]

Anchor	X	у	C _{-x}	C+x	C _{-y}	C _{+y}
1	-1.575	-1.575	-	-	-	-
2	1.575	-1.575	-	-	-	-
3	-1.575	1.575	-	-	-	-
4	1.575	1.575	-	-	-	-

Input data and results must be checked for conformity with the existing conditions and for plausibility! PROFIS Engineering (c) 2003-2021 Hilti AG, FL-9494 Schaan Hilti is a registered Trademark of Hilti AG, Schaan



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8 Remarks; Your Cooperation Duties

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